



# A Preliminary Reduced Three Dimensional Model of Regular Diagrid Tall Buildings for Fast Seismic Analysis

M. Naguib, Ahmed A. Ghaleb, S. Lotfy and Ahmed M. Yousef

**KEYWORDS:**  
 Diagrid system;  
 Reduced model;  
 Equivalent lateral force;  
 Dynamic response spectrum;  
 Linear time history.

**Abstract**— This paper introduces a reduced 3D diagrid model to save both time and computer memory for modeling and seismic analysis. The analysis of different models of steel diagrid tall buildings with variable heights and inclinations of diagrid is performed applying a finite element program SAP2000. Three clusters of diagrid buildings with different aspect ratios (height-to-width) and reliable optimum inclination angles are examined. The cross sections for diagrid elements are designed using stiffness-based design method which is more reliable for diagrid buildings. The 3D finite element model as a real structure and the proposed reduced seismic model that reducing finite element model up to twenty percent by canceling a specified number of floors with restrictions in modeling are employed. Equivalent lateral Force, dynamic response spectrum, and linear time history analyses are considered. A comparison of obtained results in terms of lateral displacement at top of the building, first mode period of vibration and base shear are presented. The conclusions reveal that the proposed reduced 3D model is convince for preliminary modeling and analysis of regular diagrid buildings in existence of seismic loads.

## I. INTRODUCTION

THE development of structural engineering has become more effective in the present decade. The fast development of tall buildings and software to analyze

these buildings invent new systems for lateral load resistance. One of the innovations complex systems is the diagrid system which gets the concerns of architectures and civil engineers. The main issue for the system is how to handle the complicated steel diagrid structures which have free-form in shape. The system operates as an integrated single unit. Diagrids are a design strategy that can resist gravity and, lateral loads through the triangulated framework shape members which eliminate vertical columns requirements. The main idea of the diagrid is the possible saving due to the removal of vertical columns. This system gives a large open area, and charming exposure. Diagrid structure is modeled as a vertical cantilever and subdivided longitudinally into a triangular diagrid module according to the repetitive diagonal

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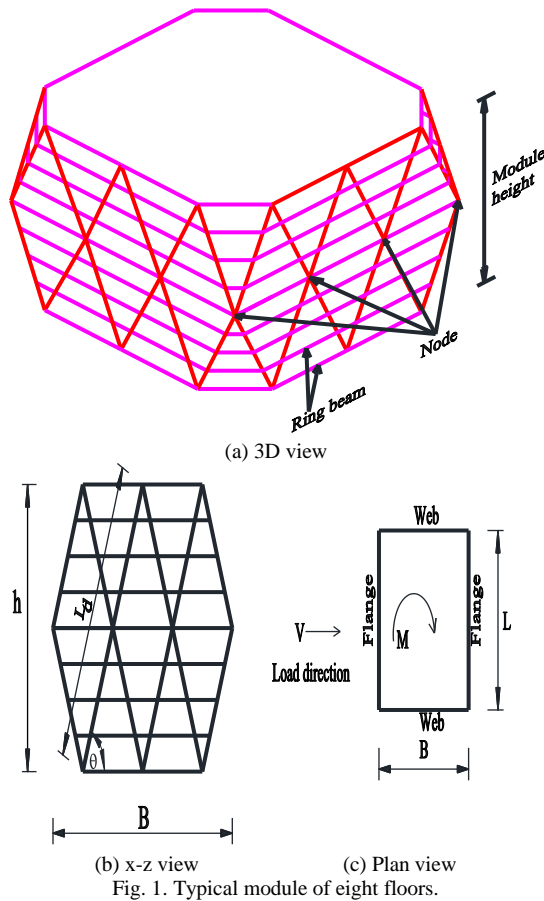
**Corresponding author:** M. Naguib, Emeritus professor in Department of Structural Engineering, Faculty of Engineering, Mansoura University, Mansoura, Egypt. (e-mail: Naguib\_2017@mans.edu.eg)

Ahmed A. Ghaleb, Emeritus Associate professor in Department of Structural Engineering, Faculty of Engineering, Mansoura University, Mansoura, Egypt.

S. Lotfy, Assistant lecturer in Civil Engineering Department, Misr Higher Institute for Engineering and Technology, Mansoura, Egypt.

Ahmed M. Yousef, professor in Department of Structural Engineering, Faculty of Engineering, Mansoura University, Mansoura, Egypt.

pattern. Each diagrid module is a single level of diagonals that elongate to a number of stories as shown in Fig. 1.



Some studies on the diagrid buildings including the diagrid inclination angle, the node connections, and seismic performance factors are indicated. The optimal angle of diagrid elements was studied by [1]–[5]. The results showed that the optimal angle of high-rise diagrid buildings falls within  $55^\circ$  to  $75^\circ$ . A coupling of optimization and robustness methods for the preliminary design of steel diagrid tall buildings using the principle of virtual work were proposed by [6]. A lot of researchers examined the creation and development of the node connections in diagrid structures [7]–[10]. Some studies on the seismic performance factors for diagrid buildings were conducted by [11]–[14]. A new method for determining seismic performance factors of steel diagrid framed systems for a number of archetypes with different parameters are examined by [11]. The proposed method reduced the efforts to compute the response modification factor based on nonlinear static analysis rather than the iterative nonlinear dynamic time history analysis. The interior core is not required to resist lateral load in the diagrid system [3]. Building height is an effective parameter on the optimum angle of diagonals [15]. As the lever arm of peripheral diagonal columns, the diagrid structural system is more effective in lateral load resistance [16]. Diagrid systems can improve the performance against seismic loads based on ductility, strength and stiffness [17].

The main objective of this paper is to propose a reduced 3D model of regular diagrid tall buildings for quick seismic analysis. Three clusters of diagrid buildings with various aspect ratios and different diagrid inclination angle  $60.94^\circ$ ,  $67.38^\circ$ ,  $71.56^\circ$  measured from the horizontal are considered for the analysis. All selected clusters were introduced by [5]. Equivalent lateral force, dynamic response spectrum, and linear time history analyses have been performed to evaluate the applicability to use the reduced seismic model in the preliminary analysis.

## II. VERIFICATION MODEL

In order to verify the suggested mathematical model, a thirty six story steel diagrid building by [16] is simulated using SAP2000. The model is shown in Fig. 2. The cross sections per IS:800-2007 [18] are shown in Table 1. The lateral seismic load and the lateral displacement along the height of the building are shown in Figs. 3, and 4. It can be seen that, good agreement is achieved, so this model is accurate enough to conduct the seismic analysis.

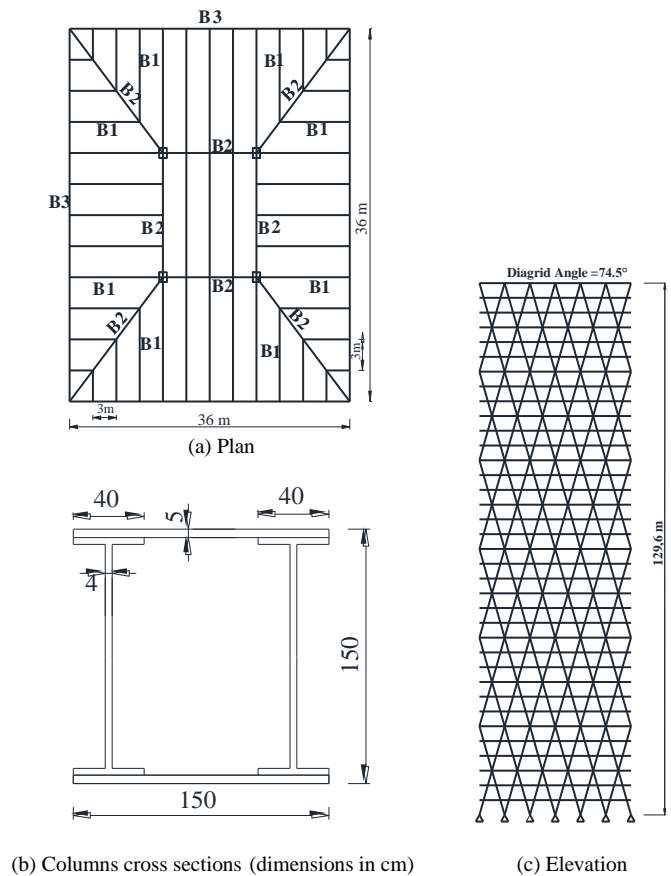


Fig. 2. Configuration for 36-story diagrid building

Table 1  
Various cross sections for thirty-six story diagrid building

Stories	Diagrid (d×t)	Columns	Beams
1-18	45 cm × 2.5 cm	150 cm × 150 cm Built up columns	B1 = B3 = ISMB 550 B2 = ISWB 600 with top and bottom Cover plate of 22 cm × 5 cm
19-36	37.5 cm × 1.2 cm		

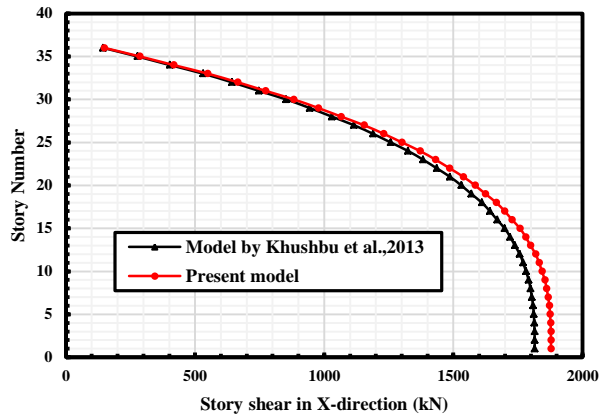


Fig. 3. Story shear along height of 36-story diagrid building.

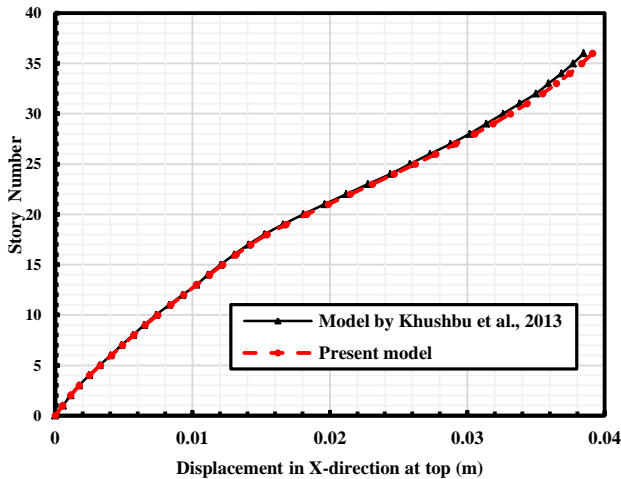


Fig. 4. Lateral displacement a long height of 36-story diagrid building

### III. METHODOLOGIES

#### A. Three- dimension Model

Three-dimensional modeling of diagrid buildings is essential because of the global behavior which cannot be simplified. It presents a more acceptable behavior to the real structure but, it needs more time for modeling and analysis with enlargement in internal memory of computers. In three-dimension modeling, the beams, columns and diagonals are treated as line elements and the slabs as shell elements. The structure is exposed to complete seismic load and resist the load by the gross stiffness and mass.

#### B. Reduced Diagrid Model

In this paper, the simplified shear beam model that introduced by [19] was followed and a simplified diagrid model is proposed by defining an equivalent stiffness and mass for diagrid elements, as shown in Fig. 5. This approach reduces the modeling time and calculations steps. The equivalent stiffness is computed herein based on the formula of springs in series as follows

$$\frac{1}{k_{eq}} = \frac{1}{k_1} + \frac{1}{k_2} + \frac{1}{k_3} \quad (1)$$

The mass is automatically modified for the slabs and beams based on the cancelled floors. Since, the mass for columns and diagrids are modified based on the following equation

$$\frac{1}{m_{eq}} = \frac{1}{m_1} + \frac{1}{m_2} + \frac{1}{m_3} \quad (2)$$

Since, the seismic load depends on the mass; the reduced load is automatically modified with the reduction of the mass.

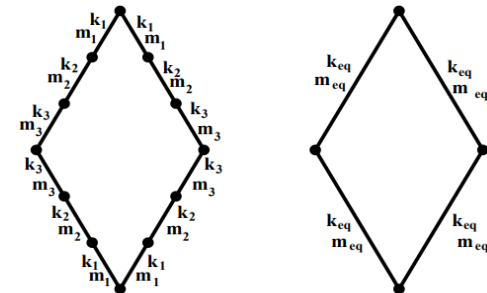


Fig.5. Equivalent Stiffness and mass for diagrid element

In order to simplify the model, the procedures are as follows; numbers of floors are reduced to be a half, third, quarter, or fifth of the total numbers. The condition for different reduced floors is shown in Table 2. The stiff solution of slabs is modeled by constrained the slab as rigid diaphragm. The equivalent stiffness and mass for various elements is defined by factoring the stiffness and the mass modifier in finite element modeling (based on property/stiffness modifiers [20]). The length of vertical columns and diagonal columns are varying increased from two to five times of the original element according to the cancelled number of floors. The seismic load is automatically modified according to the reduction of the introduced equivalent mass. The results of base shear and internal forces are magnified to more equalize the original model by a factor equal to (no of cancelled floors +1). Figures 6 to 8 show samples of the reduced modules for diagrid.

TABLE 2  
Condition for different reduced models

No of floors in half module	No of cancelled floors			
	1	2	3	4
Two	√	×	×	×
Three	×	√	×	×
Four	×	×	√	×
Five	×	×	×	√

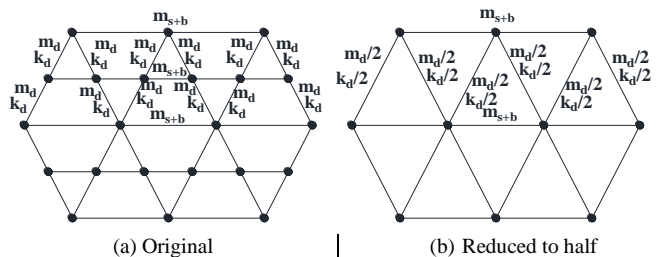


Fig. 6. Typical module of four floors

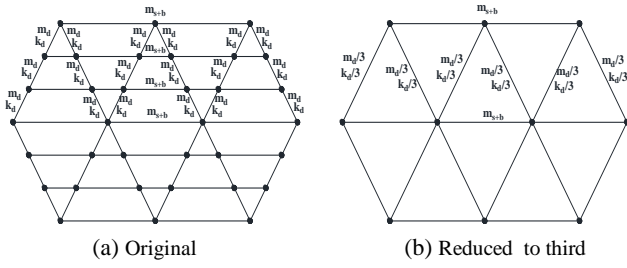


Fig. 7. Typical module of six floors

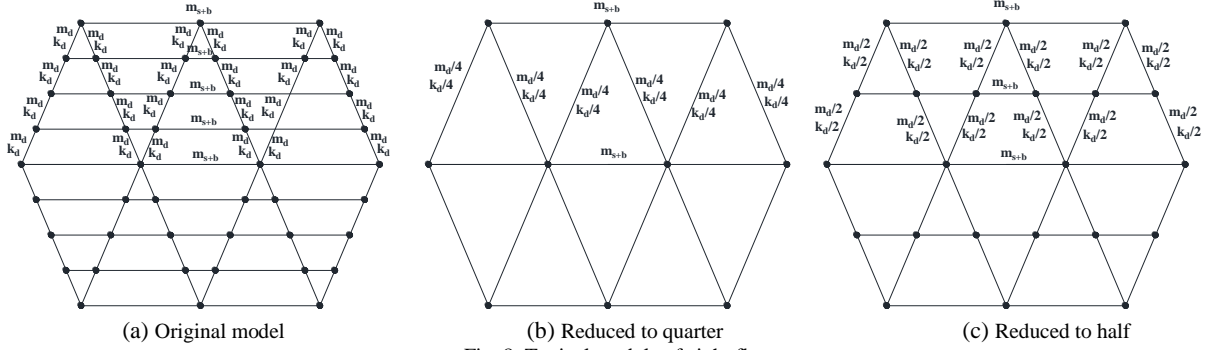


Fig. 8. Typical module of eight floors

#### IV. FINITE ELEMENT MODEL

Finite element models are considered using SAP2000. Two types of elements are used to simulate beams, diagonals, columns, and slabs. The simulation of beams, columns, and diagonals is done by using frame element (line element). The frame element has six degrees of freedom which is branched to three translations and three rotations. The simulation of slabs is done by using shell element (plane element). The shell element has six degrees of freedom which is branched to three translations and three rotations. All base joints are restrained as fixed in all directions. The methods for modeling are: three dimensional modeling which is referenced as original model and reduced 3D model which is referenced as reduced model. The original and the reduced three-dimension module of eight floors are shown in Fig. 9. The analysis is done using a workstation PC with processor (Intel (R) Xeon (R) CPU E5620 @ 2.40 GHZ 2.40 GHZ (2 processors)) and installed memory (RAM) 24.0 GB.

#### V. ANALYSIS MODEL STRUCTURES

Three clusters are selected to cover the optimum range of diagrid inclination angles based on the state of art for diagrid buildings. The variations in the aspect ratio and the number of floors in typical module are suggested to check the reduced model results for various configurations. All clusters have a square plan dimension of 40.0 m. Each cluster consists of six models with various aspect ratios. The first cluster has aspect ratio ranging from 6 to 3, and its typical story height is 3.0 m. The diagrid elements are connected to every 8 stories and make an inclination angle of  $60.94^\circ$  measured from the horizontal. The second cluster has aspect ratio ranging from 8 to 4, and its typical story height is 4.0 m. The diagrid elements are connected to every 8 stories and make an inclination angle of  $67.38^\circ$  measured from the horizontal. The third cluster has aspect ratio ranging from 8 to 3 and its typical story height is 4.0 m. The diagrid elements are connected to every 10 stories and make an inclination angle of  $71.56^\circ$  measured from the horizontal. The deformations have been controlled for buildings to behave linearly and elastically due to equivalent lateral seismic force mentioned in section VI. The cross sections for diagrid elements are empirically suggested using stiffness-based design according to the following formulas [1]:

$$A_{df} = \frac{M L_d (1+S) \alpha_h H}{(N_F + \delta) E B^2 h S \sin^2 \theta} \quad (3)$$

$$A_{dw} = \frac{\sqrt{L_d} (1+S) \alpha_h}{2N_w E h \cos^2 \theta} \quad (4)$$

The three clusters are shown in Fig. 10. A standard steel ASTM A500 [21]: grade B is used. The 16 inner vertical columns are square steel tubes with  $F_y = 31.72 \text{ kN/cm}^2$ . They have outer dimensions varying between  $135 \text{ cm} \times 135 \text{ cm}$  and  $50 \text{ cm} \times 50 \text{ cm}$  and wall thicknesses range between 4.6 cm,

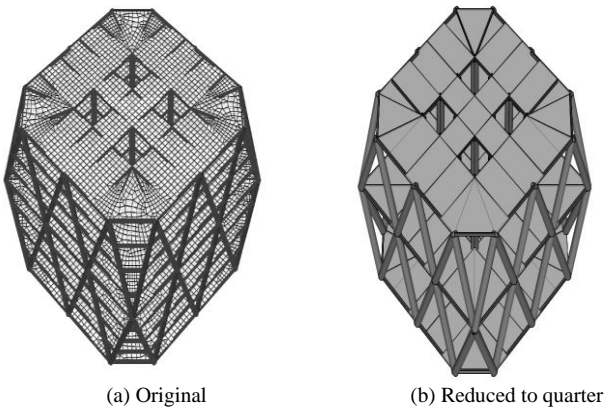
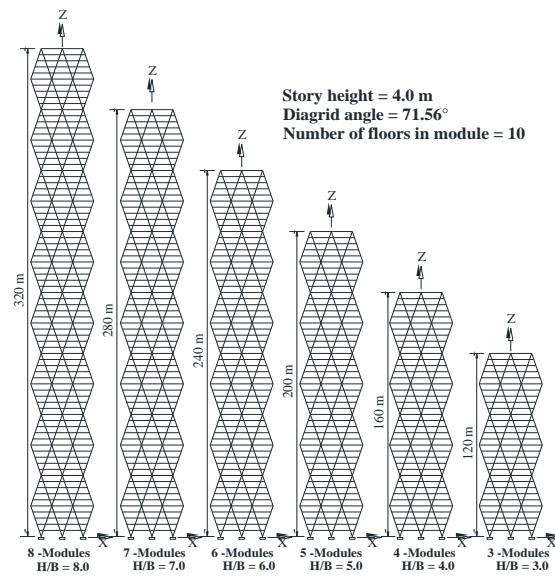
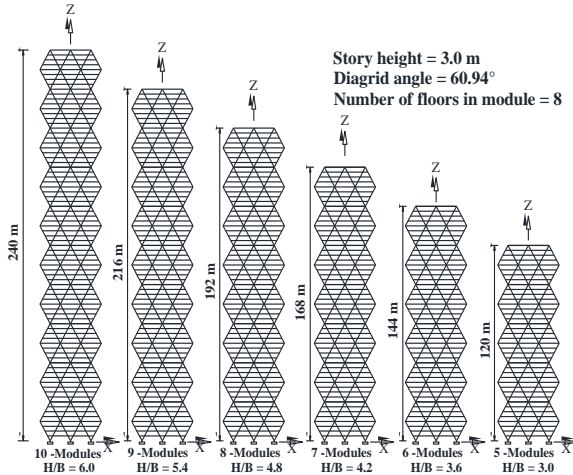


Fig. 9. Typical 3D module of eight floors

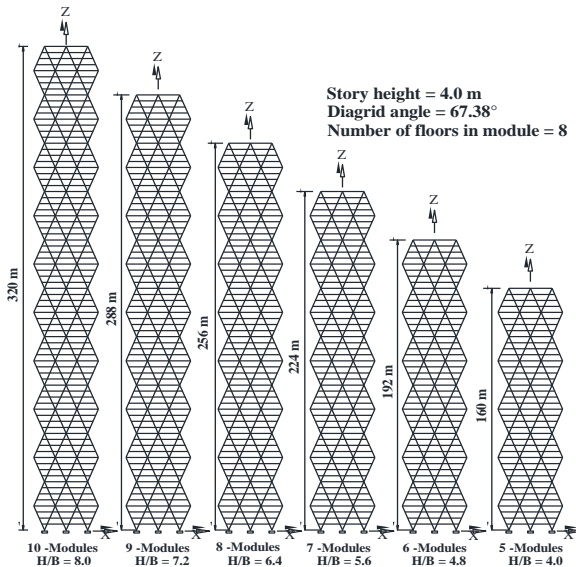
and 2.0 cm. The diagrid elements are circular steel sections with  $F_y = 28.96 \text{ kN/cm}^2$ . They have an outer-diameter varying between 179.0 cm and 40.0 cm with wall thicknesses range between 6.0 cm, and 1.3 cm. Distribution of each diagrid element cross-section along height for the three clusters is shown in Fig. 11. Building models are generated using a FORTRAN program which is constructed by [5] for importing \$2K file to SAP2000 [7]. All masses are modeled as lumped translational masses in global axes at all joints of the model. The mass is sourced from dead load plus 25% of live load. The floor slabs are defined as concrete sections with compressive strength of  $2.76 \text{ kN/cm}^2$  and thickness of 20.0 cm. All slabs (shell elements) are meshed with maximum size of 1.0 m. The used outer beams outer rectangular tube ( $30 \times 60 \text{ cm}$ ) with wall thicknesses ( $0.8 \times 1.3 \text{ cm}$ ) and the used inner beams are rectangular tube ( $20 \times 45 \text{ cm}$ ) with wall thicknesses ( $0.9 \times 1.4 \text{ cm}$ ).



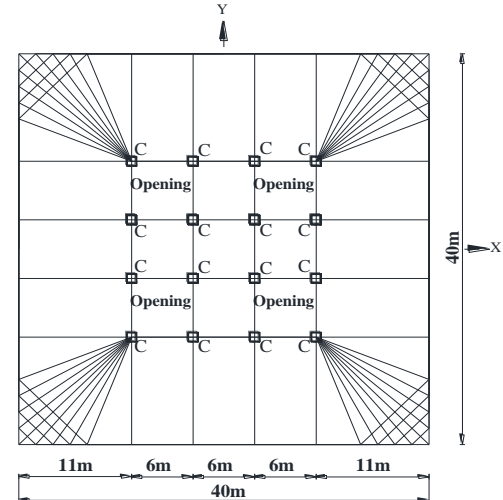
(c) X-Z view of cluster 3.



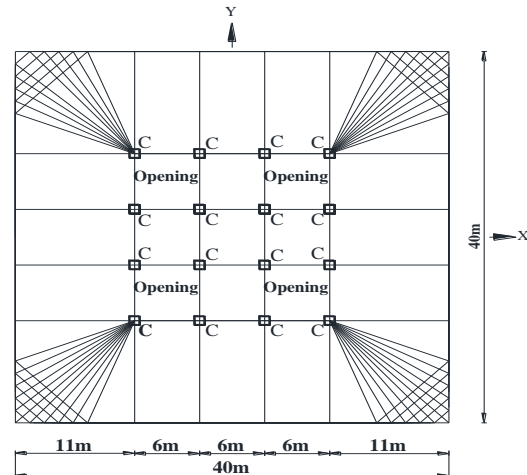
(a) X-Z view of cluster 1.



(b) X-Z view of cluster 2.



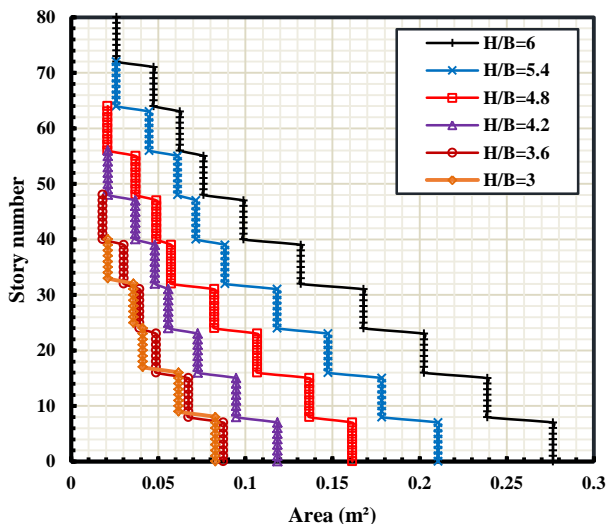
(d) Cumulative plan view of clusters 1 and 2



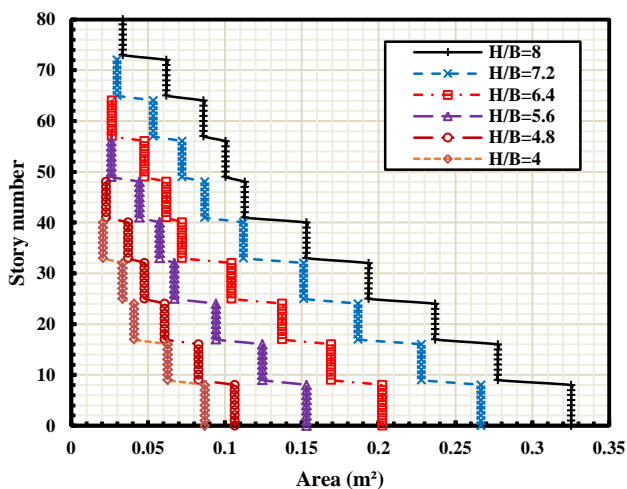
Note: C is vertical column

(e) Cumulative plan view of cluster 3.

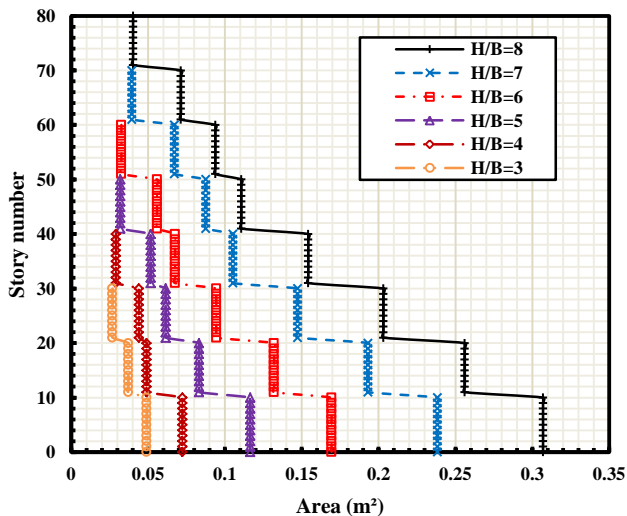
Fig. 10 Views for the three clusters



(a) Cluster 1



(b) Cluster 2



(c) Cluster 3

Fig. 11. Area of diaphragm element for each floor

VI. LOADING ASSUMPTIONS

All clusters are analyzed based on different methods of seismic analysis according to ASCE7-10 [22]. The methods are equivalent lateral force, dynamic response spectrum and time history analyses. Before seismic analysis, modal analyses are carried out for the elastic behavior with a number of modes achieving greater than or equal to 90% of mass participating ratio. All seismic loads are applied in X-direction. For the equivalent lateral force and dynamic response spectrum, the design seismic parameters are selected to match a moderate zone of earthquakes and to confirm the linear behavior as following [22]:  $S_{DS}=0.2242g$ ,  $S_{D1}=0.0816g$ ,  $C_t=0.02$ ,  $\alpha=0.75$ ,  $R=1.0$ , site class is C,  $I_e=1.0$ , and  $T_L=6$  s. For the dynamic response spectrum, the response spectrum curve is generated as shown in Fig. 12. Incorporation of the member end forces is done using the complete quadratic combination (CQC) technique.

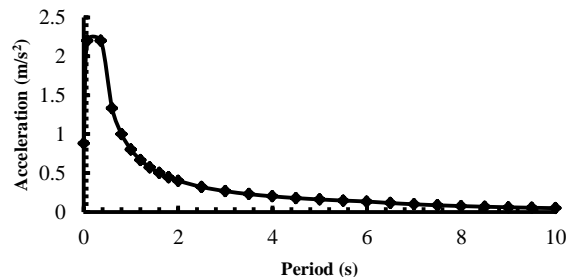


Fig. 12. Response spectrum curve

For the linear dynamic time history (Modal superposition), a Scaled El Centro acceleration record is used. El Centro earthquake record is extracted from Strong Motion Center [23] then, the baseline is corrected using SeismoSignal software [24] then, the record is scaled in the time domain to simulate the previous selected seismic parameters. ETABS [25] is used to extract the scaled El Centro record to match the same frequency content of El Centro record. Various spectrums and histories for the matching process are shown in Figs. 13 and 14. The analysis is done for 55 s with a time step of 0.02 s.

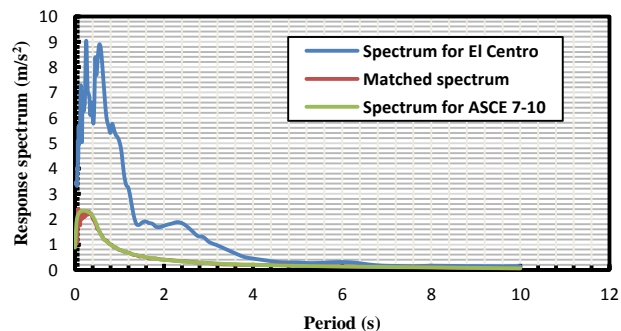


Fig. 13. Spectrum for the original and matched record

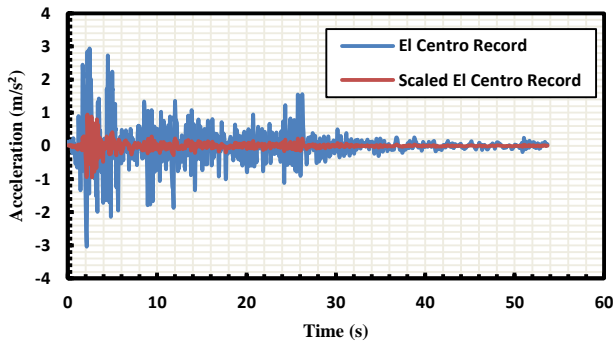


Fig. 14. Acceleration history for the original and matched record

The damping matrix is treated for dynamic response spectrum and time history analyses using Rayleigh damping equation [26] which in the form:

$$[C] = \alpha_m [M] + \beta_k [K] \tag{5}$$

Where  $\alpha_m$  and  $\beta_k$  are two constants can be computed from the following equations:

$$\alpha_m = \frac{2\xi_1 \omega_1}{2\xi_1 \omega_1 - \beta_k \omega_1^2} \tag{6}$$

$$\beta_k = \frac{\omega_1^2 - \omega_m^2}{\omega_1^2} \tag{7}$$

The first mode damping ratio is proposed by [27] for steel buildings as following:

$$\xi_1 = 1.2 + 4.26e^{-0.013H} \tag{8}$$

The higher modes damping ratios are obtained by [28] using the following equation:

$$\xi(f) = \xi_1 \left[ 1 + \gamma_m \left( \frac{f}{f_1} - 1 \right) \right] \tag{9}$$

Where;  $\gamma_m = 0.13$  for steel braced frame buildings which can be used for diagrid structures.

The Rayleigh damping needs two significant modes of vibrations. The first 13<sup>th</sup> modes of vibration satisfy the significant mass participation factor of almost 90% as shown in Table 3 for lateral translations. So, the first mode and the 13<sup>th</sup> mode are satisfactory for the computations of Rayleigh damping as shown in Table 4.

TABLE 3  
THE CUMULATIVE MASS PARTICIPATION FACTOR FOR THE FIRST THIRTEEN MODES

Cluster	H/B	The cumulative significant mass participating factor for the lateral translations X and Y	
		X-direction (%)	Y-direction (%)
1	6	89.7338	89.7338
	5.4	90.2329	90.2329
	4	90.4109	90.4109
	4.2	90.986	90.986
	3.6	91.2411	91.2411
2	3	91.3072	91.3072
	8	91.9525	91.9525
	7.2	91.8044	91.8078
	6.4	92.2407	92.2407
	5.6	92.5206	92.5206
3	4.8	92.8081	92.8081
	4.2	92.5115	92.5118
	8	91.9861	91.9861
	7	92.1882	92.1882
	6	92.4055	92.4055
3	5	92.7594	92.7593
	4	92.6883	92.6883
	3	92.3523	92.3523

TABLE 4  
DAMPING PARAMETERS FOR ALL MODELS.

Cluster	Aspect ratio	$f_1$ (Hertz)	$f_{13}$ (Hertz)	$\xi_1$	$\xi_{13}$	$\alpha$	$\beta$
1	6.0	0.152898	2.005533	1.39	3.58	0.0216	0.00546
	5.4	0.165590	2.008229	1.46	3.57	0.0244	0.005505
	4.8	0.181620	2.02333	1.55	3.59	0.0282	0.005473
	4.2	0.202281	2.043768	1.68	3.67	0.0338	0.005511
	3.6	0.229522	2.075374	1.86	3.8	0.0419	0.005476
2	3.0	0.30016	2.117161	2.1	4.02	0.054	0.00573
	8.0	0.116711	1.712913	1.27	3.53	0.0152	0.006429
	7.2	0.129968	1.719728	1.3	3.37	0.0172	0.006091
	6.4	0.142109	1.74877	1.35	3.33	0.0194	0.005901
	5.6	0.158877	1.812241	1.43	3.36	0.0228	0.005725
3	4.8	0.17734	1.837507	1.55	3.44	0.0274	0.005754
	4.0	0.214289	1.965475	1.73	3.57	0.0365	0.00542
	8.0	0.116249	1.446684	1.27	3.16	0.0149	0.006778
	7.0	0.132188	1.527470	1.31	3.11	0.0174	0.006292
	6.0	0.147507	1.536984	1.39	3.09	0.0205	0.00618
3	5.0	0.170101	1.638429	1.52	3.23	0.0256	0.006034
	4.0	0.198747	1.690392	1.73	3.42	0.0336	0.006142
	3.0	0.259629	1.870904	2.1	3.79	0.0524	0.006064

VII. ANALYSIS OF RESULTS

For comparison between the original (three dimensional) and reduced model, four methods are used in the analysis as:

- 1-Modal (free vibration)
- 2-Equivalent lateral force (ELF)
- 3-Dynamic response spectrum (RSP)
- 4-Linear time history (LTH)

The heights of the analyzed buildings are ranged from 120.0 m to 320.0 m with inclinations of diagrid ranged between 60.94° and 71.56°.

The results showed that,

- With reference to Table 5:

For the first method (modal analysis),

A good agreement is noticed between the original and reduced models for the first mode period of vibration. The error ranges between 2.36% and -14.02% for all buildings.

- With references to Fig. 15 and Table 6:

For the second method (equivalent lateral force),

The top displacement error between the original and reduced models ranges from 5.59% to -2.08% with building heights of 120.0m to 240.0m in case of half reduction. For quarter reduction, it ranges from 3.64% to -7.39%. For comparison between two models the results showed that the base shear error ranges from -0.15% to -0.32% (half reduction) and -1.12 % to -1.33% (quarter reduction).

The top displacement error between the original and reduced models ranges from 5.15% to -3.63% with building height of 160.0m to 320.0m in case of half reduction. For quarter reduction, it ranges from 9.7% to -1.79%. For comparison between two models the results showed that the base shear error ranges from -0.29% to -0.34% (half reduction) and -1.29% to -1.36 % (quarter reduction).

The top displacement error between the original and reduced models ranges from 3.17% to 19.02% with building height of 120.0m to 320.0m in case of fifth reduction. For comparison between two models the results showed that the base shear error ranges from -0.15% to -0.32% in case of fifth reduction.

The equivalent lateral force is a simple method to perform the seismic analysis but, it neglects the contributions of the higher modes.

- With references to Fig. 16 and Table 7:

For the third method (dynamic response spectrum),

The top displacement error between the original and reduced models ranges from 1.42% to -0.6% with building height of 120.0m to 240.0m in case of half reduction. For quarter reduction, it ranges from 4.02% to -2.63%. For comparison between the original and reduced models the results showed that the base shear error ranges from 1.95% to -12.01% (half reduction) and 7.77% to -7.95% (quarter reduction).

The top displacement error between the original and reduced models ranges from 0.61% to -1.85% with building height of 160.0m to 320.0m in case of half reduction. For quarter reduction, it ranges from 2% and -3.76%. For comparison between the original and reduced models the results showed that the base shear error ranges from -0.12% to 3.96% (half reduction) and 0.9% to 3.39% (quarter reduction).

The top displacement error between the original and reduced models ranges from 0.42% to -5.31% with building height of 160.0m to 320.0m in case of half reduction. For quarter reduction, it ranges from 4.28% to -14.98%. For comparison between two models the results showed that the base shear error ranges from 4.28% to -14.98% (half reduction) and 12.23% to -22.75 % (quarter reduction).

The top displacement error between the original and reduced models ranges from 14.95% to -15.96% with building height of 120.0m to 320.0m in case of fifth reduction. For comparison between two models the results showed that:

The top displacement error at between the original and reduced models ranges from 3.11% to -9.13% with building height of 120.0m to 320.0m in case of fifth reduction. For comparison between two models the results showed that the base shear error ranges from 5.26% to 17.07% in case of fifth reduction. The response spectrum is a good seismic approach to predict the maximum responses in the linear zone. It includes the contributions of the higher modes for the maximum response.

- With references to Figs. 17 to 19 and Table 8:

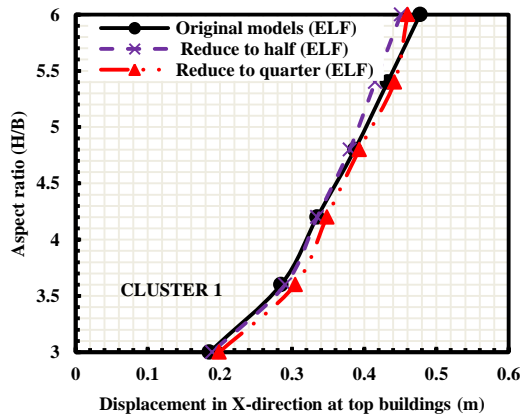
For the fourth method (linear time history),

The top displacement error between the original and reduced models ranges from 7.39% to -9.23% with building height of 120.0m to 240.0m in case of half reduction. For quarter reduction, it ranges from 5.65% to -16.2%. For comparison between two models the results showed that the base shear error ranges from 16.02% to -10.09% (half reduction) and 16.57% to -14.11% (quarter reduction).

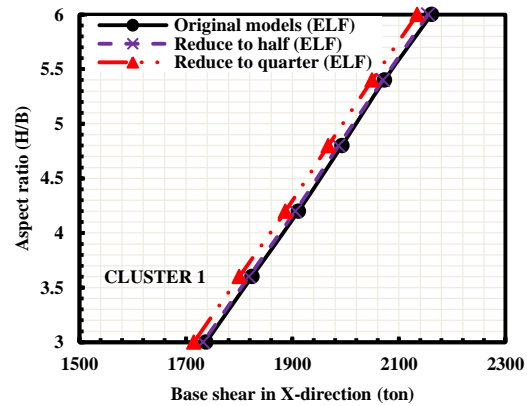
base shear error ranges from 25% to 16.89% in case of fifth reduction.

The linear time history could predict the behavior of the buildings along the time.

The obtained results can give an overview for the behavior of diagrid buildings with less effort in modeling and analysis rather than the 3D complex detailed modeling. The reduced model can give an approximation on the modes of vibration, the lateral deformations and initial checks on the selected cross sections based on initial internal force and the overall flexibility of the model.



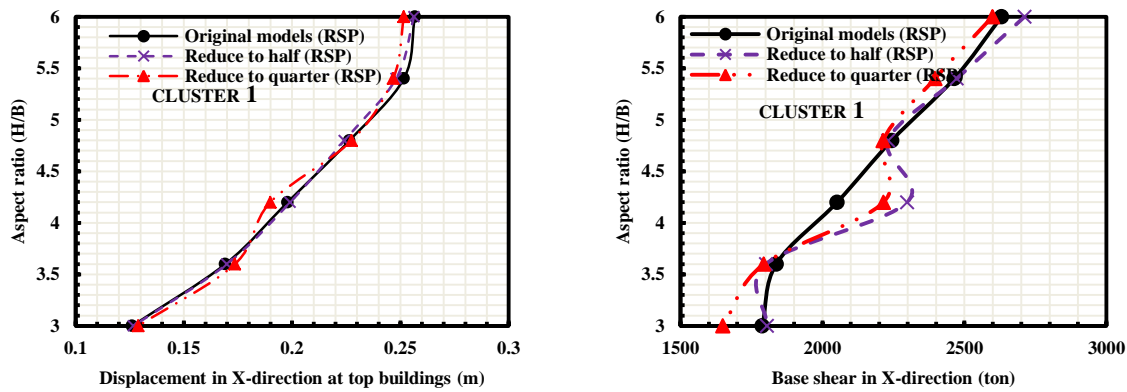
(a) Lateral displacement at top buildings



(b) Lateral base shear

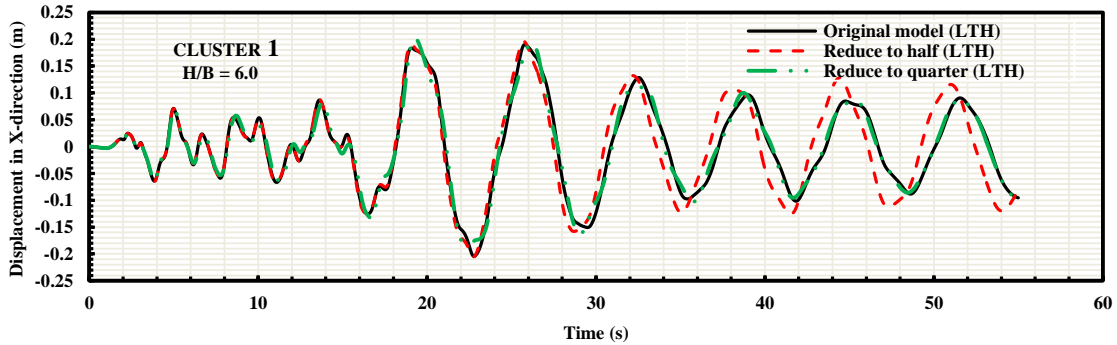
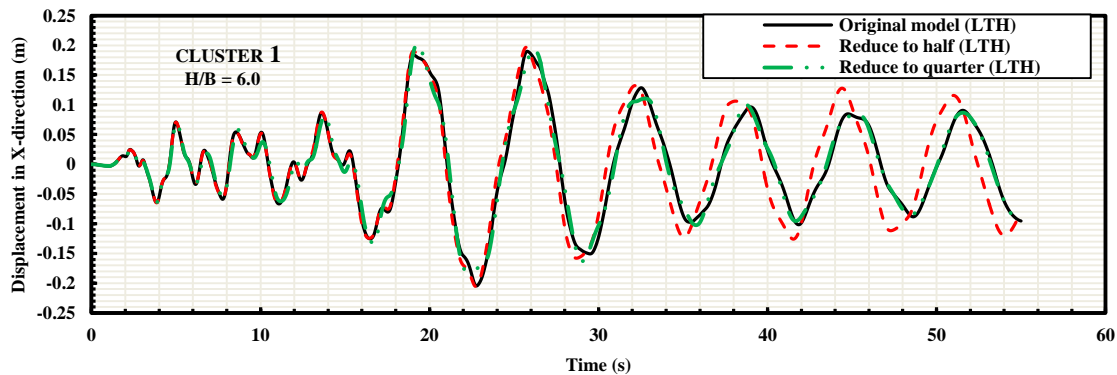
Fig. 15. Cluster 1: top displacement and base shear (ELF)



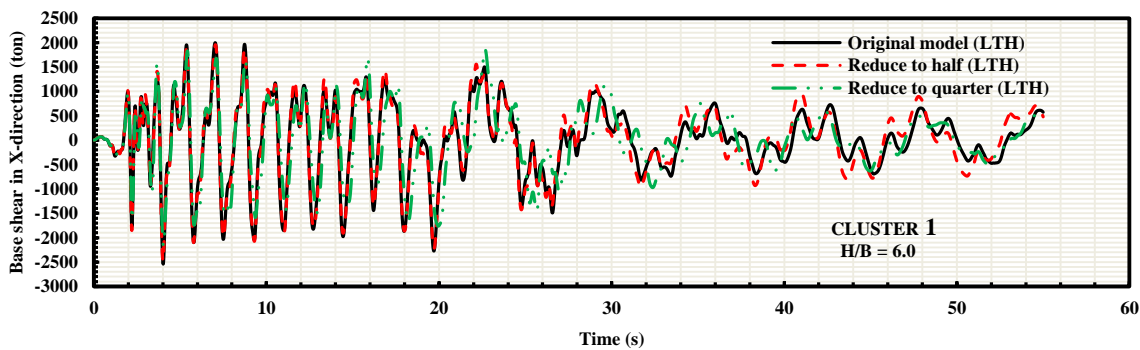


(a) Lateral displacement at top buildings (b) Lateral base shear

Fig. 16. Cluster 1: top displacement and base shear (RSP)

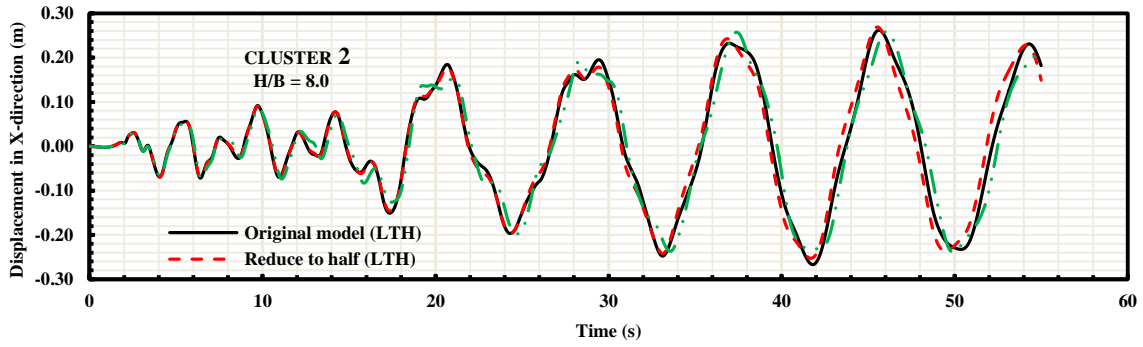


(a) Lateral displacement history at top building

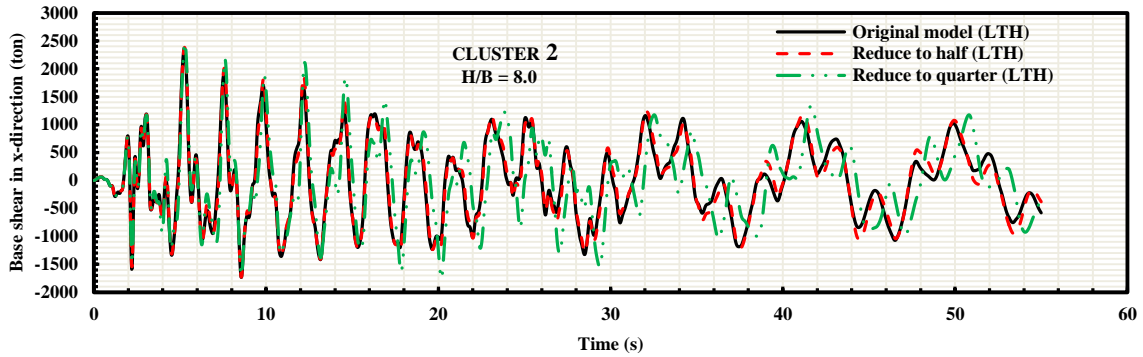


(b) Lateral base shear history

Fig. 17. Cluster 1: Building with height of 240m (LTH)

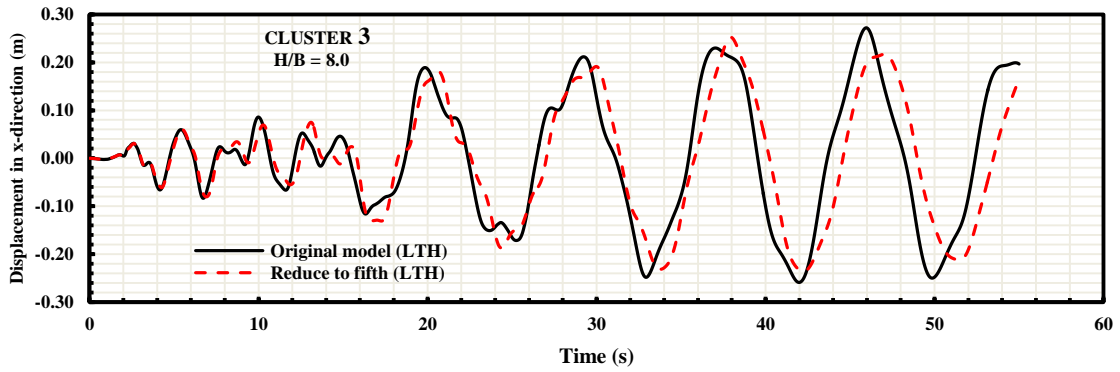


(a) Lateral displacement history at top building

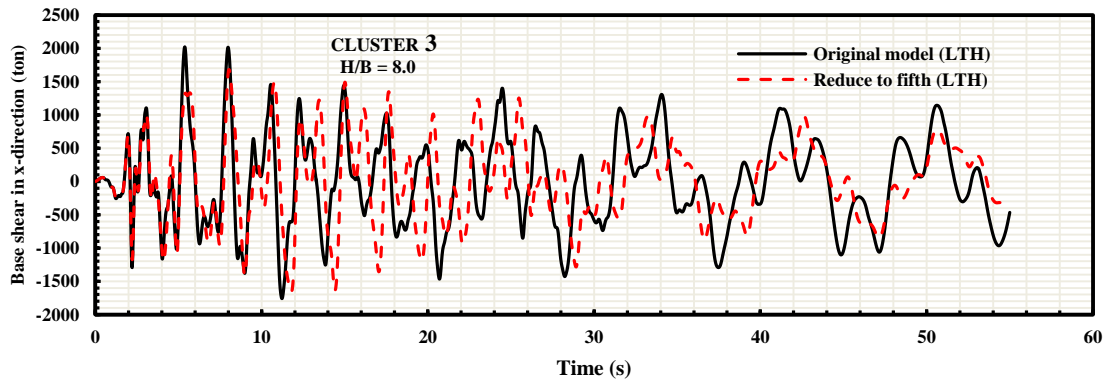


(b) Lateral base shear history

Fig. 18. Cluster 2: Building with height of 320m (LTH)



(a) Lateral displacement history at top building



(b) Lateral base shear history

Fig. 19. Cluster 3: Building with height of 320m (LTH)

TABLE 5  
First Mode Period of Vibration for All Buildings

Cluster 1				Cluster 2				Cluster 3		
H/B	Original (s)	Reduce to half (s)	Reduce to quarter (s)	H/B	Original (s)	Reduce to half(s)	Reduce to quarter (s)	H/B	Original (s)	Reduce to Fifth (s)
6	6.54	6.39	6.53	8	8.57	8.46	8.64	8	8.60	8.94
5.4	6.04	5.95	6.22	7.2	7.69	7.63	7.84	7	7.56	7.95
4.8	5.51	5.49	5.66	6.4	7.04	7.04	7.25	6	6.78	7.24
4.2	4.94	4.98	5.15	5.6	6.29	6.35	6.61	5	5.88	6.38
3.6	4.36	4.44	4.62	4.8	5.64	5.75	5.98	4	5.03	5.58
3	3.33	3.40	3.55	4	4.67	4.83	5.02	3	3.85	4.39

TABLE 6  
DIVERGENCE BETWEEN THE ORIGINAL AND REDUCED MODELS FOR (ELF)

Aspect ratio (H/B)	Cluster 1 (Error%)				Aspect ratio (H/B)	Cluster 2 (Error%)				Aspect ratio (H/B)	Cluster 3 (Error%)	
	Reduce to half		Reduce to quarter			Reduce to half		Reduce to quarter			Reduce to fifth	
	Disp. at top	Base shear	Disp. at top	Base shear		Disp. at top	base shear	Disp. at top	base shear		Disp. at top	base shear
6	5.59	-0.30	3.64	-1.24	8	-3.63	-0.33	-1.79	-1.29	8	3.17	-1.52
5.4	3.99	-0.15	-2.21	-1.12	7.2	-2.75	-0.29	-0.05	-1.32	7	5.10	-1.63
4.8	1.83	-0.31	-1.64	-1.29	6.4	-1.21	-0.34	1.98	-1.36	6	8.12	-1.53
4.2	-0.01	-0.31	-4.02	-1.28	5.6	0.26	-0.34	5.45	-1.35	5	10.66	-1.52
3.6	-2.02	-0.32	-6.93	-1.30	4.8	2.23	-0.33	6.98	-1.34	4	14.47	-1.50
3	-2.08	-0.32	-7.39	-1.33	4	5.15	-0.33	9.70	-1.36	3	19.02	-1.52

TABLE 7  
DIVERGENCE BETWEEN ORIGINAL MODELS AND SIMPLE MODELS FOR (RSP)

Aspect ratio (H/B)	Cluster 1 (Error%)				Aspect ratio (H/B)	Cluster 2 (Error%)				Aspect ratio (H/B)	Cluster 3 (Error%)	
	Reduce to half		Reduce to quarter			Reduce to half		Reduce to quarter			Reduce to fifth	
	Disp. at top%	Base shear%	Disp. at top%	Base shear%		Disp. at top%	base shear%	Disp. at top%	base shear%		Disp. at top%	base shear%
6	0.16	-3.08	1.97	1.21	8	0.61	-0.12	2.00	0.90	8	2.62	2.39
5.4	1.42	-0.37	1.97	2.76	7.2	0.77	1.99	2.20	0.48	7	2.79	2.77
4.8	1.04	0.56	-0.46	1.34	6.4	0.83	2.59	2.13	1.53	6	3.11	3.01
4.2	-0.60	-12.01	4.02	-7.95	5.6	1.09	3.93	2.85	3.39	5	1.22	5.26
3.6	-0.44	1.95	-2.63	2.37	4.8	-0.71	2.94	-2.67	2.87	4	-6.66	4.26
3	-0.44	-0.94	-2.27	7.77	4	-1.85	3.96	-3.76	2.40	3	-9.13	17.07

TABLE 8  
DIVERGENCE FOR THE MAXIMUM ABSOLUTE VALUES BETWEEN ORIGINAL MODELS AND SIMPLE MODELS FOR (SCALED EL CENTRO).

Aspect ratio (H/B)	Cluster 1 (Error%)				Aspect ratio (H/B)	Cluster 2 (Error%)				Aspect ratio (H/B)	Cluster 3 (Error%)	
	Reduce to half		Reduce to quarter			Reduce to half		Reduce to quarter			Reduce to fifth	
	Disp. at top%	Base shear%	Disp. At top%	Base shear%		Disp. at top%	base shear%	Disp. at top%	base shear%		Disp. at top%	base shear%
6	-0.48	2.20	1.32	14.95	8	-0.77	-1.38	3.06	-4.91	8	7.25	16.89
5.4	-1.58	-0.34	1.69	16.54	7.2	-0.98	-12.02	-9.02	-17.20	7	-15.96	9.77
4.8	-4.84	-9.36	-6.24	-14.11	6.4	0.42	-14.98	-5.35	-22.75	6	7.18	8.52
4.2	7.39	16.02	5.65	16.57	5.6	-0.20	4.28	7.38	5.33	5	14.32	9.65
3.6	-9.23	-8.74	-16.20	-7.06	4.8	-3.78	1.86	-7.75	-0.81	4	-9.53	25.00
3	-6.04	-10.09	-2.70	1.59	4	-5.31	0.28	-2.80	12.23	3	14.95	5.45

VIII. CONCLUSION

The results showed that,

- The using of reduced model instead the three dimensional model had a significant effect in all obtained results saving in the degrees of freedom and time executed.
- The equivalent lateral force is the best methods in comparison with other mentioned method in this paper because is independent on the natural frequencies for higher modes.
- The amount of reduction percentage may affect the results.

- A good agreement is noticed in case of reduction to 50%.
- All obtained results are used as a guide for preliminary analysis to reach the best representative for real structure. Then, the exact three dimensions for structures must take into consideration to make the best analysis.

### Nomenclature

Aspect ratio	The ratio between building height and width
$K_{eq}$	Equivalent stiffness for the reduced model
$m_{eq}$	Equivalent mass for the reduced model
$m_{s+b}$	Total mass of slab and beams for considered story
$m_d$	Mass of diagrid element
$K_d$	Stiffness for diagrid element
M	The moment at level of consideration
$L_d$	The diagrid member length
S	The ratio between bending and shear deformations at top of the building
$\alpha_h$	A factor used for the allowable lateral displacement at top constrained to 500
H	The Building height
$N_F$	The number of diagonals on flange side
$\delta$	An estimate of the contribution of the diagonals on each web to the bending rigidity is made by adding one extra diagonal on each flange;
E	The modulus of elasticity
B	The plan dimension of the building parallel to the considered lateral load direction
h	The module height
$\theta$	The angle of diagrid elements
V	The shear force at level of consideration
$N_w$	The number of diagonals on web side
$N_f$	The number of diagonals on flange side
$S_{DS}$	Design earthquake spectral response acceleration parameter at short period.
$S_{D1}$	Design earthquake spectral response acceleration parameter at 1 second.
G	Gravity acceleration.
$C_t$	Building period coefficient.
x	Constant depends on structure type.
R	Response modification factor.
$I_e$	Importance factor.
$T_L$	Long period transition.
[C]	Damping matrix of the structure.
[M]	Mass matrix of the structure.
[K]	Stiffness matrix of the structure.
$\alpha_m$	Mass constant for Rayleigh damping.
$\beta_k$	Stiffness constant for Rayleigh damping.
$\xi_1$	Damping ratio for the first mode.
$\xi_m$	Damping ratio for the $m^{\text{th}}$ significant higher mode considered for the analysis.
$\xi(f)$	Damping ratio used for the higher modes.
$\gamma_m$	Constant depends on structural system.
$f_n$	Natural frequency for the $n^{\text{th}}$ significant mode.
$f_1$	Natural frequency for the first mode.
ELF	Equivalent lateral force.
RSP	Dynamic response spectrum.
LTH	Linear time history.

### AUTHORS CONTRIBUTION

Mohamed Naguib Abou El-Saad, Ahmed Amin Ghaleb and Ahmed Mahmoud Yousef conceived of the presented

idea. Sameh Lotfy Mahmoud developed the theory and performed the computations and analyses. Mohamed Naguib Abou El-Saad, Ahmed Amin Ghaleb verified the methods of analysis. Ahmed Amin Ghaleb and Mohamed Naguib Abou El-Saad have supervised the findings of this work. All authors discussed the results and contributed to the final manuscript. Sameh Lotfy Mahmoud wrote the manuscript with support from Mohamed Naguib Abou El-Saad

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#### **Title Arabic:**

(نموذج مبدني مختزل ثلاثي الأبعاد لمباني الشبكات المائلة العالية للتحليل الزلزالي السريع)

#### **Arabic Abstract:**

يقدم هذا البحث نموذج مختزل ثلاثي الأبعاد للشبكات المائلة لتوفير الوقت وذاكرة الحاسب للتمثيل والتحليل الزلزالي وقد تم تحليل نماذج مختلفة من مباني الشبكات المائلة العالية والتي لها ارتفاعات وميول مختلفة من الشبكات المائلة بواسطة برنامج SAP2000 وقد تم عمل دراسة على ثلاثة مجموعات من مباني الشبكات المائلة مع نسب ارتفاع الى عرض مختلفة وميول مثالية للشبكات المائلة وتم تصميم القطاعات لهذه الشبكات بواسطة طريقة الصلابة والتي تعتبر ملائمة لهذا النوع من المنشآت وقد تم استخدام طريقتين للنمذجة وهما النمذجة ثلاثية الأبعاد التي تحاكي نموذج العناصر المحددة لتكون مشابهة تقريباً للهيكل الحقيقي والنموذج الزلزالي المختزل الذي يحاكي اختزال نموذج العناصر المحددة حتى الخمس عن طريق حذف عدد من الطوابق مع قيود في النمذجة و تم الأخذ في الاعتبار عدة عوامل وهي القوة الجانبية المكافئة و طيف الاستجابة الديناميكي و تحليلات السجل الزمني الخطي و تم مقارنة النتائج من حيث الزمن الدوري الأسلسى والازاحة الجانبية القصوى عند سطح المبنى وقوى القص عند الأساسات. وأكدت النتائج أن النموذج المختزل الذي تم اقتراحه هو مناسب للتحليل المبدني لأنظمة الشبكات المائلة العالية نتيجة الأحمال الزلزالية.